

A Simplified Method for Designing RC Slabs under Concentrated Loading

Morteza Fadaee, Atefeh Iranmanesh, and Mohammad J. Fadaee

Abstract—Designing two-way slabs under transverse concentrated loads is one of the challenges in designing such structures. No certain method can be found in the design codes or references for designing two-way slabs subjected to concentrated loads. However, plastic method can be the best method for designing such cases of two-way slabs. This method requires various formulas for different conditions of RC slab and so, is practically not simple and not usable in designing offices. In this paper, plastic method is used for different conditions of RC slabs and also various dimensions, in order to derive the essential formulas followed by calculating the positive and negative moment coefficients for concentrated loading located in different points of a two-way slab. The efficiency of this method is shown by a numeric example at the end of the paper.

Index Terms—Concentrated loading, plastic method, RC two-way slab, yield lines.

I. INTRODUCTION

Accurate analysis of two-way slabs having different continuity conditions at their edges is very difficult and for practical purposes is almost impossible. Thus, there are several simplified methods for determining the moments, shear forces and support reactions [1]-[2]. The methods used for designing RC members are mostly based on elastic analysis of the structure subjected to the ultimate loads though, the actual behavior of an indeterministic structure is that when one or more member reach its bending resistance, the elastic diagrams of this part will change to some extent and so, the elastic analysis results cannot be used anymore. In such case if the structure has sufficient ductility, each time that the section reaches its bending resistance, the bending moments will redistribute until some plastic hinges or plastic lines form and, the structure becomes unstable. In such circumstances, the structure cannot resist any more load and collapses (see Fig. 1). Such type of analysis in which the bending moment diagrams at the failure point are used as a basis for the design, is called plastic analysis [3]. Although there are some other methods for analyzing and designing RC slabs subjected to concentrated load such as finite elements, finite difference methods, and also using the plate's theory method, but still the best and most practical available method is the plastic method.

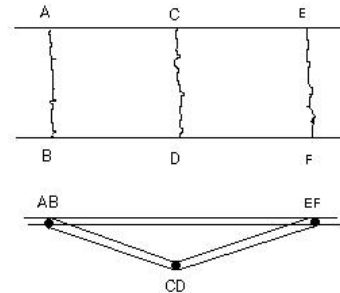


Fig. 1. Failure mechanism in a slab.

In plastic method which is also known as the yield lines method, it is assumed that the resistance of the slab is determined by bending only, and the other factors such as shear or displacement should be considered separately.

It must be noted that in the yield lines method the behavior of the slab can be considered better. Many of slab systems which cannot be analyzed with other methods such as equivalent frame method, direct method and moment coefficients method because of the slab specific shape or loading type, can be designed by plastic method. In fact, by the plastic method it is possible to design any type of slabs with any shape subjected to any type of loadings.

Yield lines method is used for slabs which are reinforced at any direction, which means that the section area of the reinforcement is assumed constant per unit length, although the value of the reinforcement can vary in the upper face or the bottom face of the slab. It is also possible to use the yield line analysis for slabs with non-uniform reinforcement distribution, though it may be more difficult [3].

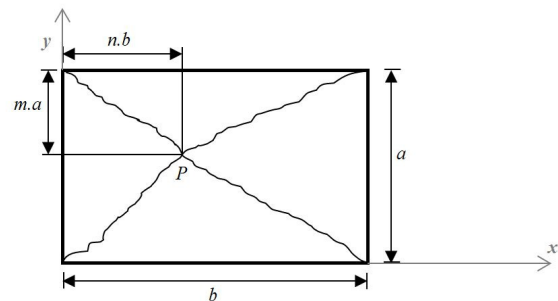


Fig. 2. Triangular failure schema for concentrated load.

II. YIELD LINE PATTERN FOR CONCENTRATED LOAD

In the case of existence of a concentrated load, the schema of failure may include the yield lines around of the load. The failure schema that contains the curved yield lines for the negative moment and the radial yield lines for the positive

Manuscript received April 15, 2013; revised July 2, 2013.

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moment would be more critical than the failure schema with triangular segments among the yield lines [1]. However, in this paper the triangular failure schema is used for the concentrated load and the calculations are based on this type of schema, (see Fig. 2).

III. EQUATIONS FOR MOMENT COEFFICIENTS CALCULATION

In this section, the relations of the plastic method for the rectangular slabs having simple or fixed supported edges are obtained. For this purpose, the internal work is considered equal to the external work based upon the following equation,

$$\sum m_{ux} \theta_x l_y + \sum m_{uy} \theta_y l_x = P \delta \quad (1)$$

where m_{uy} , m_{ux} are moments per unit length about the x and y axes, respectively, θ_y, θ_x are the yield lines' rotation angles about the x and y axes, respectively, l_y, l_x are the yield lines' lengths along the x and y axes, respectively, P is the concentrated load and δ is the displacement under the application point of the concentrated load. In this work, the slab is assumed to be isotropic, therefore, $m_{ux} = m_{uy} = m_u$. For a rectangular slab with simple supported edges, the following relation can be obtained from (1),

$$m_u = \frac{P}{\frac{b}{ma} + \frac{a}{nb} + \frac{b}{a(1-m)} + \frac{a}{b(1-n)}} \quad (2)$$

In which m_u is the positive moment per unit length along the yield lines and the other parameters are defined as in Fig. 2. If it is assumed that $m_u = f.P$, then the factor f can be found from the following equation,

$$f = \frac{1}{\frac{1}{mr} + \frac{r}{n} + \frac{1}{r(1-m)} + \frac{r}{(1-n)}} \quad (3)$$

where, $r = \frac{a}{b}$. For rectangular slabs with fixed supported edges, (1) results in the following equation,

$$m_u^+ + m_u^- = \frac{P}{\frac{b}{ma} + \frac{a}{nb} + \frac{b}{a(1-m)} + \frac{a}{b(1-n)}} \quad (4)$$

In which, m_u^- is the negative moment per unit length for the fixed supported edges and m_u^+ is the positive moment per unit length. In this paper it is assumed that the reinforcement in the slab is such that the slab edges can resist negative moment about 1.5 times of the positive moment ($m_u^- = 1.5m_u^+$). So, (4) results in,

$$f = \frac{1}{2.5(\frac{1}{mr} + \frac{r}{n} + \frac{1}{r(1-m)} + \frac{r}{(1-n)})} \quad (5)$$

By similar calculations the relations needed for other conditions of the slab edges, can be determined.

IV. INTRODUCING DIFFERENT SLAB EDGES CONDITIONS

In this paper different conditions of the slab edges are taken the same as are defined in [4] numbered 1 to 9. In all of these conditions of the slab edges, the four edges are supported and the fixing conditions only are different (see Fig. 3).

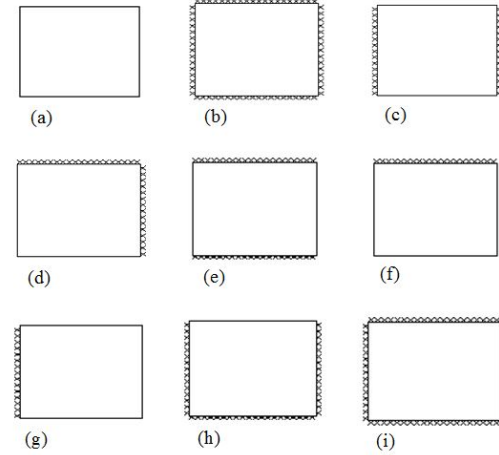


Fig. 3. Different slab edges conditions.

Case 1: None of the edges are fixed (see Fig. 3-a).

Case 2: All of the four edges are fixed (see Fig. 3-b).

Case 3: The two shorter edges only are fixed (see Fig. 3-c).

Case 4: The two adjacent edges only are fixed (see Fig. 3-d).

Case 5: The two longer edges only are fixed (see Fig. 3-e).

Case 6: One of the longer edges only is fixed (see Fig. 3-f).

Case 7: One of the shorter edges only is fixed (see Fig. 3-g).

Case 8: One of the longer edges only is *not* fixed (see Fig. 3-h).

Case 9: One of the shorter edges only is *not* fixed (see Fig. 3-i).

V. MOMENT COEFFICIENTS TABLES

Base upon the equations provided in the previous section, the values of moment coefficient are calculated for various ratios of dimensions of two-way slabs from 0.55 to 1, concentrated load located in different positions of the slab, and different conditions of the edges supports as shown in the Tables I to IV as samples. Similar tables can be prepared for the other cases. The location of the concentrated load is defined by the parameters m and n as shown in Fig. 2.

It must be noted that the factors calculated for parameters m and n in the tables are in values of 0.1 to 0.5 only, because the moments for intervals of 0.5 to 1 are the same, regarding geometry of the slab.

VI. NUMERICAL EXAMPLE

A rectangular RC slab having dimensions of $8.5m \times 5.5m$ for which only two adjacent edges are fixed supported is subjected to a concentrated load of $900kN$ at a point as shown in Fig. 3. In this example, the weight of the slab is not accounted for. Reinforcements for positive and negative

moments at the edges are calculated as follows.

as follows,

Parameters needed for calculating the factor f can be found

TABLE I: CONCENTRATED LOAD COEFFICIENTS FOR SLAB CASE 1

m	r	0.55	0.6	0.65	0.7	0.75	0.8	0.85	0.9	0.95	1
n	f										
0.1	0.1	0.038	0.0397	0.0411	0.0423	0.0432	0.0439	0.0444	0.0448	0.0449	0.045
	0.2	0.0423	0.0449	0.0473	0.0494	0.0513	0.0529	0.0544	0.0556	0.0567	0.0576
	0.3	0.0438	0.0468	0.0495	0.0521	0.0544	0.0565	0.0584	0.0601	0.0617	0.063
	0.4	0.0445	0.0476	0.0505	0.0532	0.0557	0.0581	0.0602	0.0621	0.0639	0.0655
	0.5	0.0446	0.0478	0.0508	0.0536	0.0561	0.0585	0.0607	0.0627	0.0645	0.0662
0.2	0.1	0.0572	0.0585	0.0594	0.0599	0.06	0.0599	0.0595	0.059	0.0584	0.0576
	0.2	0.0676	0.0706	0.0731	0.0752	0.0768	0.078	0.079	0.0796	0.0799	0.08
	0.3	0.0715	0.0753	0.0787	0.0816	0.084	0.086	0.0877	0.089	0.0901	0.0908
	0.4	0.0732	0.0774	0.0811	0.0844	0.0873	0.0897	0.0918	0.0935	0.0949	0.096
	0.5	0.0737	0.078	0.0819	0.0853	0.0882	0.0908	0.093	0.0948	0.0963	0.0976
0.3	0.1	0.0677	0.0685	0.0687	0.0686	0.0681	0.0674	0.0665	0.0654	0.0642	0.063
	0.2	0.0827	0.0856	0.0878	0.0895	0.0906	0.0913	0.0916	0.0916	0.0913	0.0908
	0.3	0.0887	0.0926	0.096	0.0987	0.1008	0.1024	0.1036	0.1044	0.1049	0.105
	0.4	0.0913	0.0958	0.0997	0.1029	0.1055	0.1077	0.1094	0.1106	0.1115	0.112
	0.5	0.0921	0.0967	0.1007	0.1041	0.107	0.1093	0.1111	0.1125	0.1135	0.1141
0.4	0.1	0.0731	0.0735	0.0734	0.0728	0.072	0.0709	0.0697	0.0684	0.0669	0.0655
	0.2	0.0908	0.0935	0.0955	0.0968	0.0976	0.098	0.0979	0.0975	0.0969	0.096
	0.3	0.0981	0.102	0.1052	0.1077	0.1096	0.1109	0.1117	0.1122	0.1122	0.112
	0.4	0.1013	0.1059	0.1097	0.1128	0.1152	0.1171	0.1184	0.1193	0.1198	0.12
	0.5	0.1023	0.107	0.111	0.1143	0.1169	0.1189	0.1205	0.1215	0.1222	0.1224
0.5	0.1	0.0747	0.075	0.0748	0.0741	0.0732	0.072	0.0707	0.0692	0.0677	0.0662
	0.2	0.0934	0.096	0.0979	0.0991	0.0998	0.1	0.0998	0.0993	0.0985	0.0976
	0.3	0.1011	0.105	0.1081	0.1105	0.1123	0.1135	0.1142	0.1145	0.1145	0.1141
	0.4	0.1046	0.1091	0.1128	0.1159	0.1182	0.12	0.1212	0.122	0.1224	0.1224
	0.5	0.1056	0.1103	0.1142	0.1174	0.12	0.122	0.1234	0.1243	0.1248	0.125

TABLE II: CONCENTRATED LOAD COEFFICIENTS FOR SLAB CASE 2

m	r	0.55	0.6	0.65	0.7	0.75	0.8	0.85	0.9	0.95	1
n	f										
0.1	0.1	0.0152	0.0159	0.0164	0.0169	0.0173	0.0176	0.0178	0.0179	0.018	0.018
	0.2	0.0169	0.018	0.0189	0.0198	0.0205	0.0212	0.0218	0.0223	0.0227	0.023
	0.3	0.0175	0.0187	0.0198	0.0208	0.0218	0.0226	0.0234	0.0241	0.0247	0.0252
	0.4	0.0178	0.019	0.0202	0.0213	0.0223	0.0232	0.0241	0.0249	0.0256	0.0262
	0.5	0.0179	0.0191	0.0203	0.0214	0.0225	0.0234	0.0243	0.0251	0.0258	0.0265
0.2	0.1	0.0229	0.0234	0.0238	0.0239	0.024	0.024	0.0238	0.0236	0.0233	0.023
	0.2	0.027	0.0282	0.0292	0.0301	0.0307	0.0312	0.0316	0.0318	0.032	0.032
	0.3	0.0286	0.0301	0.0315	0.0326	0.0336	0.0344	0.0351	0.0356	0.036	0.0363
	0.4	0.0293	0.031	0.0325	0.0338	0.0349	0.0359	0.0367	0.0374	0.038	0.0384
	0.5	0.0295	0.0312	0.0327	0.0341	0.0353	0.0363	0.0372	0.0379	0.0385	0.039
0.3	0.1	0.0271	0.0274	0.0275	0.0274	0.0272	0.027	0.0266	0.0262	0.0257	0.0252
	0.2	0.0331	0.0342	0.0351	0.0358	0.0362	0.0365	0.0366	0.0366	0.0365	0.0363
	0.3	0.0355	0.0371	0.0384	0.0395	0.0403	0.041	0.0415	0.0418	0.0419	0.042
	0.4	0.0365	0.0383	0.0399	0.0412	0.0422	0.0431	0.0437	0.0442	0.0446	0.0448
	0.5	0.0368	0.0387	0.0403	0.0417	0.0428	0.0437	0.0444	0.045	0.0454	0.0457
0.4	0.1	0.0292	0.0294	0.0293	0.0291	0.0288	0.0284	0.0279	0.0273	0.0268	0.0262
	0.2	0.0363	0.0374	0.0382	0.0387	0.0391	0.0392	0.0392	0.039	0.0387	0.0384
	0.3	0.0392	0.0408	0.0421	0.0431	0.0438	0.0444	0.0447	0.0449	0.0449	0.0448
	0.4	0.0405	0.0424	0.0439	0.0451	0.0461	0.0468	0.0474	0.0477	0.0479	0.048
	0.5	0.0409	0.0428	0.0444	0.0457	0.0468	0.0476	0.0482	0.0486	0.0489	0.049
0.5	0.1	0.0299	0.03	0.0299	0.0296	0.0293	0.0288	0.0283	0.0277	0.0271	0.0265
	0.2	0.0373	0.0384	0.0392	0.0396	0.0399	0.04	0.0399	0.0397	0.0394	0.039
	0.3	0.0404	0.042	0.0432	0.0442	0.0449	0.0454	0.0457	0.0458	0.0458	0.0457
	0.4	0.0418	0.0436	0.0451	0.0463	0.0473	0.048	0.0485	0.0488	0.049	0.049
	0.5	0.0422	0.0441	0.0457	0.047	0.048	0.0488	0.0493	0.0497	0.0499	0.05

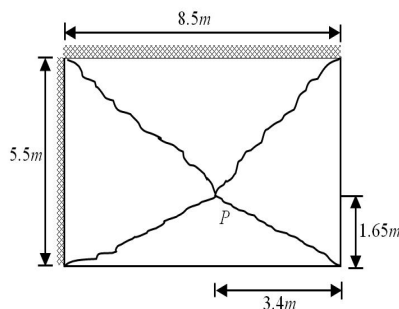


Fig. 4. Numerical example.

$$m = \frac{1.65}{5.5} = 0.3, \quad n = \frac{3.4}{8} = 0.4, \quad r = \frac{5.5}{8.5} \approx 0.65$$

So, the factor f from the Table IV is determined as 0.0574, thus,

$$f = 0.0574 \Rightarrow m_u^+ = 0.0574 \times 900 = 51.66 \frac{kN.m}{m}$$

As the negative moment is assumed 1.5 times of the positive moment, therefore, the negative moment per unit length is calculated as follows,

TABLE III: CONCENTRATED LOAD COEFFICIENTS FOR SLAB CASE 3

	<i>r</i>	0.55	0.6	0.65	0.7	0.75	0.8	0.85	0.9	0.95	1
<i>m</i>	<i>n</i>	<i>f</i>									
0.1	0.1	0.0282	0.0284	0.0284	0.0283	0.0281	0.0277	0.0273	0.0268	0.0263	0.0257
	0.2	0.0347	0.0359	0.0367	0.0373	0.0377	0.0379	0.0379	0.0379	0.0377	0.0374
	0.3	0.0374	0.039	0.0403	0.0413	0.0421	0.0427	0.0431	0.0434	0.0435	0.0434
	0.4	0.0386	0.0404	0.0419	0.0432	0.0442	0.045	0.0456	0.046	0.0463	0.0465
	0.5	0.0389	0.0408	0.0424	0.0437	0.0448	0.0457	0.0464	0.0468	0.0472	0.0474
0.2	0.1	0.0375	0.0369	0.0361	0.0352	0.0343	0.0333	0.0323	0.0313	0.0303	0.0294
	0.2	0.0501	0.0505	0.0506	0.0503	0.0499	0.0492	0.0485	0.0476	0.0467	0.0457
	0.3	0.0558	0.0569	0.0576	0.0579	0.0579	0.0577	0.0572	0.0566	0.0559	0.0551
	0.4	0.0585	0.06	0.061	0.0617	0.0619	0.0619	0.0617	0.0613	0.0607	0.06
	0.5	0.0593	0.0609	0.0621	0.0628	0.0632	0.0632	0.0631	0.0627	0.0622	0.0615
0.3	0.1	0.0418	0.0406	0.0394	0.0381	0.0368	0.0355	0.0342	0.033	0.0318	0.0307
	0.2	0.058	0.0578	0.0572	0.0564	0.0553	0.0542	0.053	0.0517	0.0504	0.0491
	0.3	0.0658	0.0663	0.0664	0.0661	0.0655	0.0646	0.0636	0.0625	0.0613	0.06
	0.4	0.0695	0.0705	0.0709	0.071	0.0706	0.07	0.0692	0.0682	0.0671	0.0659
	0.5	0.0706	0.0718	0.0723	0.0724	0.0722	0.0717	0.0709	0.07	0.0689	0.0677
0.4	0.1	0.0438	0.0424	0.0409	0.0394	0.0379	0.0365	0.0351	0.0338	0.0325	0.0313
	0.2	0.0618	0.0613	0.0604	0.0592	0.0579	0.0565	0.055	0.0535	0.052	0.0505
	0.3	0.0708	0.071	0.0707	0.07	0.069	0.0679	0.0666	0.0652	0.0637	0.0622
	0.4	0.0752	0.0758	0.0759	0.0755	0.0748	0.0738	0.0727	0.0714	0.07	0.0686
	0.5	0.0765	0.0773	0.0775	0.0772	0.0766	0.0757	0.0746	0.0734	0.072	0.0706
0.5	0.1	0.0443	0.0429	0.0413	0.0397	0.0382	0.0367	0.0353	0.034	0.0327	0.0315
	0.2	0.063	0.0623	0.0613	0.0601	0.0586	0.0571	0.0556	0.054	0.0525	0.051
	0.3	0.0724	0.0724	0.072	0.0712	0.0701	0.0689	0.0675	0.066	0.0644	0.0629
	0.4	0.0769	0.0774	0.0774	0.0769	0.0761	0.075	0.0737	0.0724	0.0709	0.0694
	0.5	0.0783	0.0789	0.079	0.0787	0.0779	0.0769	0.0757	0.0744	0.0729	0.0714

TABLE IV: CONCENTRATED LOAD COEFFICIENTS FOR SLAB CASE 4

	<i>r</i>	0.55	0.6	0.65	0.7	0.75	0.8	0.85	0.9	0.95	1
<i>m</i>	<i>n</i>	<i>f</i>									
0.1	0.1	0.0183	0.0195	0.0206	0.0216	0.0225	0.0225	0.024	0.0247	0.0252	0.0257
	0.2	0.0261	0.0279	0.0295	0.031	0.0324	0.0324	0.0347	0.0357	0.0366	0.0374
	0.3	0.0302	0.0323	0.0342	0.0359	0.0375	0.0375	0.0403	0.0415	0.0425	0.0434
	0.4	0.0326	0.0348	0.0368	0.0386	0.0403	0.0403	0.0432	0.0444	0.0455	0.0465
	0.5	0.0339	0.0361	0.0381	0.0399	0.0416	0.0416	0.0443	0.0455	0.0465	0.0474
0.2	0.1	0.0219	0.0232	0.0244	0.0254	0.0264	0.0264	0.0279	0.0285	0.029	0.0294
	0.2	0.0339	0.036	0.0378	0.0395	0.0409	0.0409	0.0433	0.0443	0.0451	0.0457
	0.3	0.0412	0.0437	0.0459	0.0478	0.0496	0.0496	0.0523	0.0534	0.0543	0.0551
	0.4	0.0458	0.0484	0.0507	0.0528	0.0545	0.0545	0.0574	0.0584	0.0593	0.06
	0.5	0.0484	0.051	0.0532	0.0552	0.0569	0.0569	0.0594	0.0603	0.061	0.0615
0.3	0.1	0.0233	0.0246	0.0258	0.0269	0.0278	0.0278	0.0293	0.0299	0.0304	0.0307
	0.2	0.0374	0.0395	0.0414	0.0431	0.0446	0.0446	0.0469	0.0478	0.0485	0.0491
	0.3	0.0464	0.049	0.0513	0.0533	0.055	0.055	0.0576	0.0586	0.0594	0.06
	0.4	0.0522	0.055	0.0574	0.0594	0.0612	0.0612	0.0638	0.0647	0.0654	0.0659
	0.5	0.0557	0.0584	0.0606	0.0625	0.0641	0.0641	0.0663	0.067	0.0675	0.0677
0.4	0.1	0.0239	0.0252	0.0265	0.0275	0.0285	0.0285	0.0299	0.0305	0.0309	0.0313
	0.2	0.0389	0.0411	0.043	0.0447	0.0462	0.0462	0.0485	0.0493	0.05	0.0505
	0.3	0.0489	0.0515	0.0538	0.0558	0.0575	0.0575	0.06	0.061	0.0617	0.0622
	0.4	0.0554	0.0582	0.0606	0.0626	0.0643	0.0643	0.0668	0.0676	0.0682	0.0686
	0.5	0.0592	0.0619	0.0642	0.0661	0.0675	0.0675	0.0695	0.0701	0.0705	0.0706
0.5	0.1	0.0241	0.0254	0.0266	0.0277	0.0286	0.0286	0.0301	0.0307	0.0311	0.0315
	0.2	0.0394	0.0416	0.0435	0.0452	0.0467	0.0467	0.0489	0.0498	0.0504	0.051
	0.3	0.0496	0.0522	0.0545	0.0565	0.0582	0.0582	0.0608	0.0617	0.0624	0.0629
	0.4	0.0563	0.0591	0.0615	0.0635	0.0652	0.0652	0.0676	0.0684	0.069	0.0694
	0.5	0.0603	0.063	0.0653	0.0671	0.0686	0.0686	0.0705	0.071	0.0713	0.0714

$$m_u^- = 1.5m_u^+ = 1.5 \times 51.66 = 77.49 \text{ kN.m / m}$$

$$m_u^- = 77.49 \frac{\text{kN.m}}{\text{m}} \Rightarrow A_s^- = 1902.38 \frac{\text{mm}^2}{\text{m}}$$

Now the reinforcement of positive and negative moments can be determined based upon a design Code. In this example CCI (Concrete Code of Iran) [5] is used. The thicknesses of the slab h , and the effective depth d are taken as 18cm and 15cm, respectively. The concrete's compressive strength f_c and steel's yield stress f_y are 21MPa and 400MPa, respectively. Thus,

in longitudinal direction,

$$\begin{cases} A_s^+ = 9807.47 \text{ mm}^2 \Rightarrow \text{USE } \phi 18 @ 220 \text{ mm} \\ A_s^- = 16170.23 \text{ mm}^2 \Rightarrow \text{USE } \phi 24 @ 230 \text{ mm} \end{cases}$$

in latitudinal direction,

$$\begin{cases} A_s^+ = 6346.01 \text{ mm}^2 \Rightarrow \text{USE } \phi 18 @ 220 \text{ mm} \\ A_s^- = 10463.09 \text{ mm}^2 \Rightarrow \text{USE } \phi 24 @ 230 \text{ mm} \end{cases}$$

$$m_u^+ = 51.66 \frac{\text{kN.m}}{\text{m}} \Rightarrow A_s^+ = 1153.82 \frac{\text{mm}^2}{\text{m}}$$

The plan of the slab's reinforcement is shown in Fig. 5:

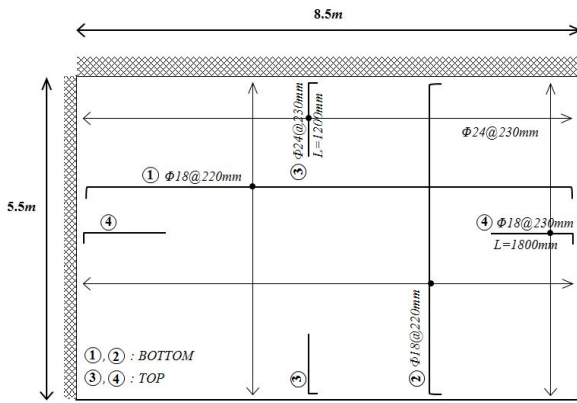


Fig. 5. Reinforcement of the example.

VII. CONCLUSION

As no practical method for designing two-way RC slabs subjected to concentrated load can be found in RC books, publications and codes, in this research using the plastic method (yield lines method) which is an appropriate method for designing the slabs subjected to concentrated load, moment coefficients for different conditions of slab edges supports and various slab dimensions and different points of application of the loadings have been provided that can be used for designing RC slabs practically at the engineering offices.

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